

Asphalt in Railways Tracks



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Asphalt in Railways Tracks

- › General overview
- › Recent HSL projects in France



- › Asphalt Mix advantages
- › Conclusion



NOTE D'INFORMATION



Réalisation d'assises de voie ferrée en grave bitume : Retour d'expérience de chantiers LGV

1 Introduction

La majeure partie du réseau ferroviaire français a été construite au cours des 19 et 20^{èmes} siècles, avec des techniques permettant la réalisation de structures à base de graves non traitées. Depuis la fin des années 90, de nouvelles techniques sont apparues remplaçant cette couche d'assise en grave non traitée par une couche de grave-bitume ferroviaire (GB F), technique issue du domaine routier. D'abord inscrite comme variante et expérimentée sur une section de quelques kilomètres au début des années 2000, cette technique s'est rapidement développée pour être retenue dans le cadre des LGV construites ces dernières années.

Afin de capitaliser sur les récents chantiers réalisés et de partager l'expérience française sur l'utilisation de ces techniques, les acteurs réunis au sein de l'IDRRIM ont souhaité porter la rédaction de cette note d'information dont l'objectif est d'effectuer un premier retour d'expérience sur l'utilisation de cette technique routière adaptée aux infrastructures ferroviaires.

Outre une approche générale et documentée par une bibliographie internationale, ce document détaillera les structures mises en œuvre, les modalités de dimensionnement de ces structures et leur mise en œuvre en vue d'identifier les avantages et inconvénients issus de l'utilisation de cette technique.



La plupart de ces projets étant relativement récents (Lignes Est Européenne (EE), Bretagne-Pays de la Loire (BPL) et Sud Europe Atlantique (SEA)), des éléments complémentaires sur le comportement de ces techniques pourront être apportés par les instrumentations en cours.

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- 2 | Structures types en comparaison
- 3 | Expériences internationales
- 4 | Expériences en France
- 5 | Synthèse
- 6 | Conclusions et perspectives
- 7 | Bibliographie

General overview

- › Road and Rail Tracks common ground :
 - ✓ Infrastructures
 - ✓ Transportation (People and Goods)
 - ✓ Important network
 - ✓ Cost (construction / maintenance)
- › Not the same requirements

Surface	Rail Tracks	Asphalt Wearing Course
Loading		
Speed	80 – 340km/h	50 – 110km/h
Design Lifetime	75 – 100 years	20 – 30 years

Asphalt in Railways : An old story...

- › Italy
 - Since 1907
 - More than 1200 km
- › Germany)
 - Several solutions (ATD, Satro, GETRAC...)
- › Japan

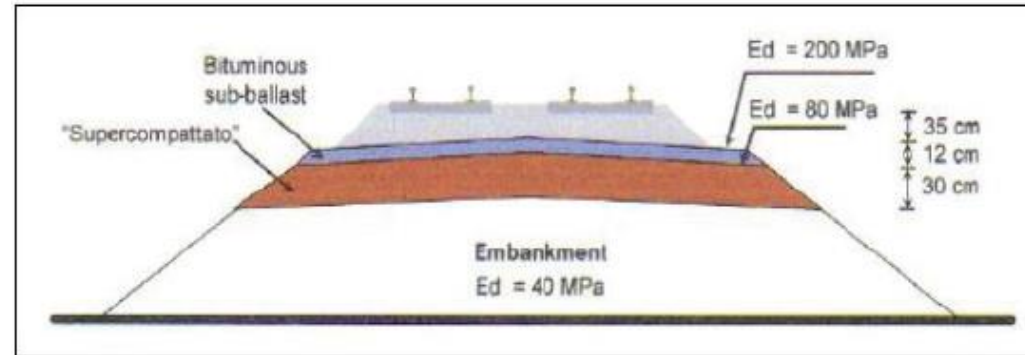


Figure 1: Italian High-Speed Railway Cross-Sectional Profile [1.]

- › USA (since 1980)

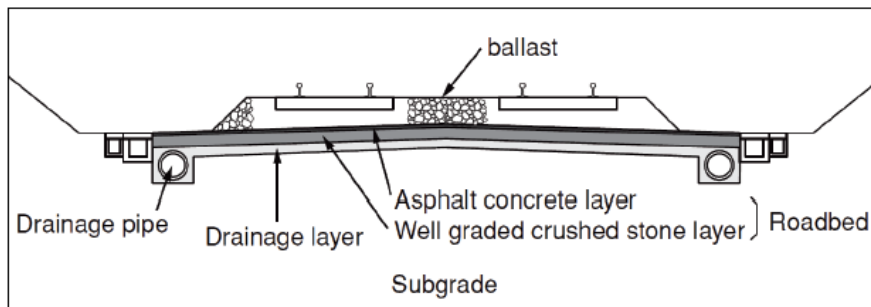


Figure 11: Cross section of ballasted track and asphalt roadbed [10.]

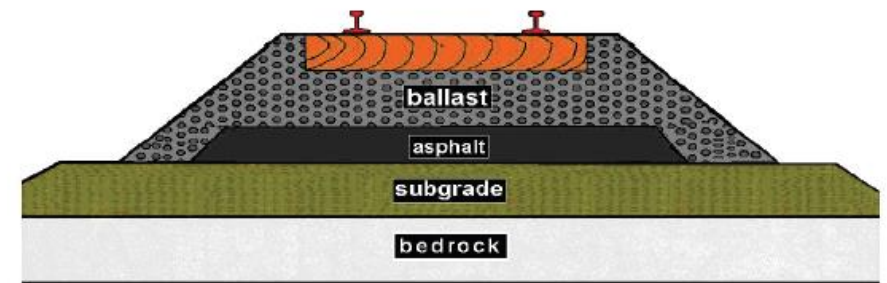
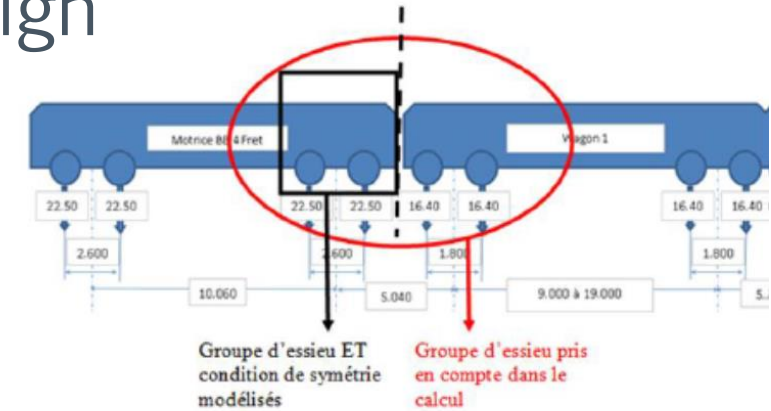
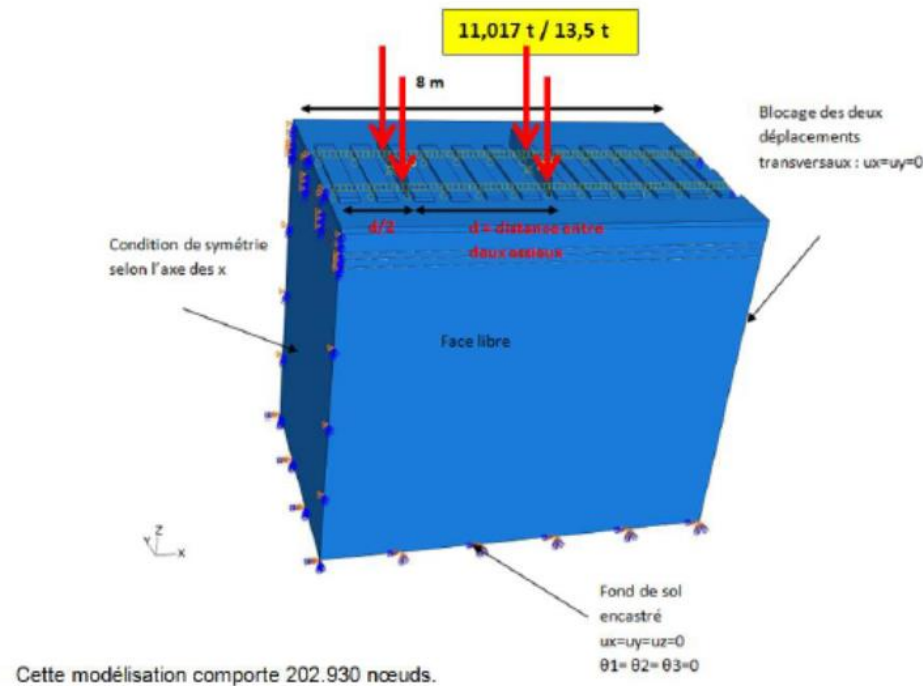


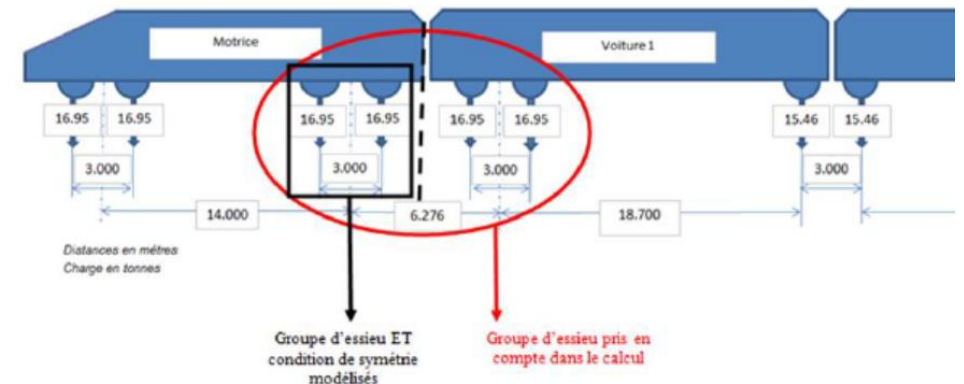
Figure 12: "Asphalt Underlayment" trackbed without granular sub-ballast layer [9.]

Evolution of the structure

- › To reduce cost / To take advantage of AC mixes
- › French Experience on HSL
- › Design : 3D modelling + Road Design



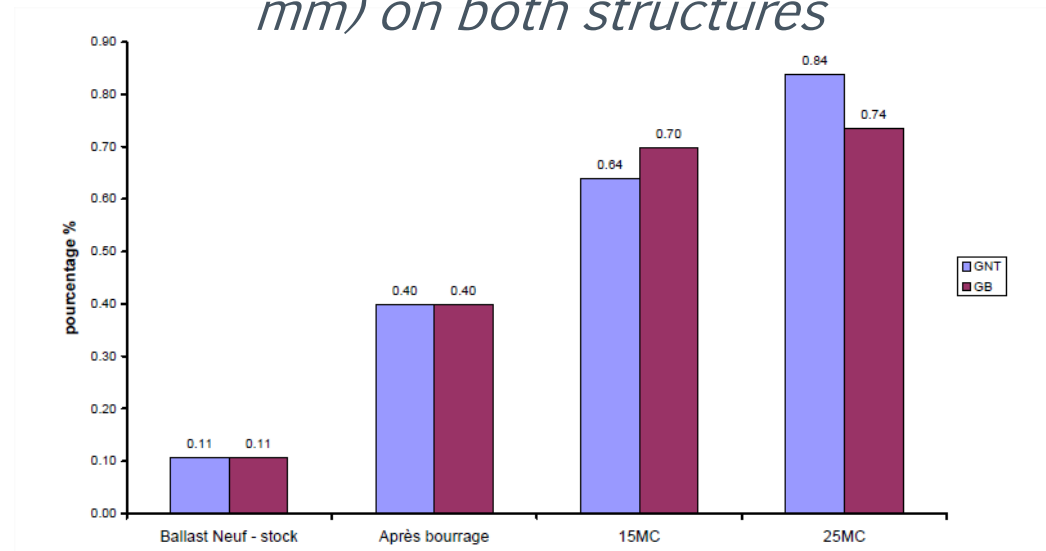
➤ TGV - High Speed Train



Impact of the Asphalt layer on the durability of the ballast

› SCALE ONE TRIALS: ATTRITION TESTS AT 25 MILLIONS CYCLES

Fine aggregates distribution (< 0.5 mm) on both structures



NO CRUSHING EFFECT OF THE ASPHALT LAYER

Key advantages of asphalt layer

› Construction phase

- Reduction of pavement layer thickness
- Reduction of Ballast extra thickness on edges
- Reduction of sensitivity to weather conditions (works schedule secured)
- Improved traffic conditions before laying the track
- Possible supply of a first layer of ballast

› Under operations

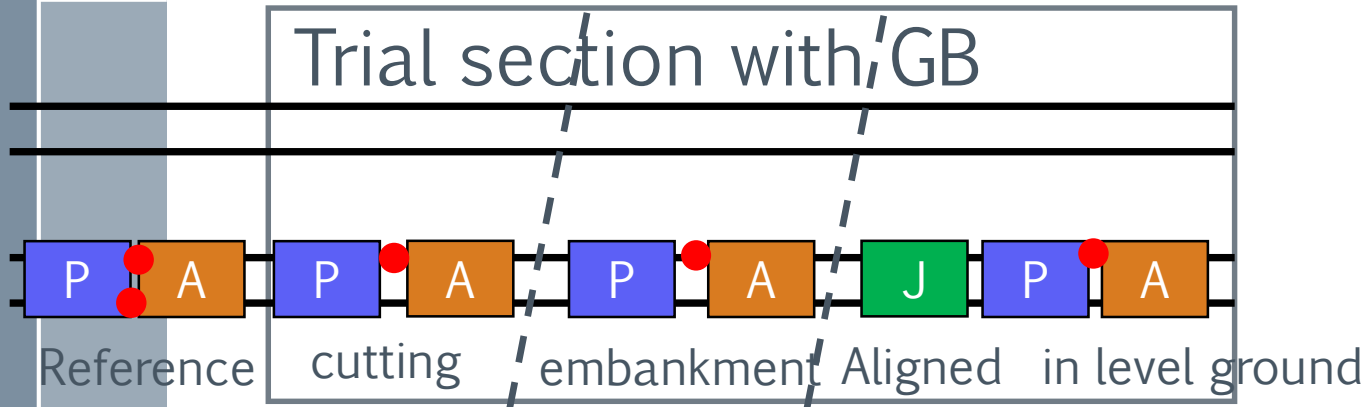
- Improved homogeneity of the platform
- Improved protection against water infiltrations
- Increase stability of the asphalt layer
- No need of phytosanitary products against vegetation





First Trial on HSL: East-European HSL

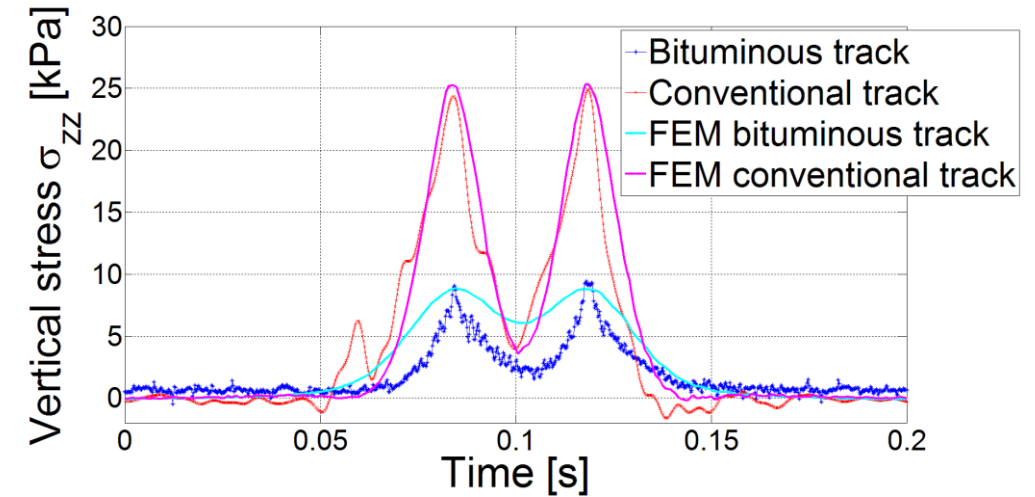
- › 3km experimental test section (2005-2007)
- › Near Reims (slopes, structure, bridges, cut and fill)
- › One section monitored



First Trial on HSL: East-European HSL



-  Accelerometer
-  Pressure under traverse stresses in GNT strain measurements
-  Strain gauges in GB
-  Temperature



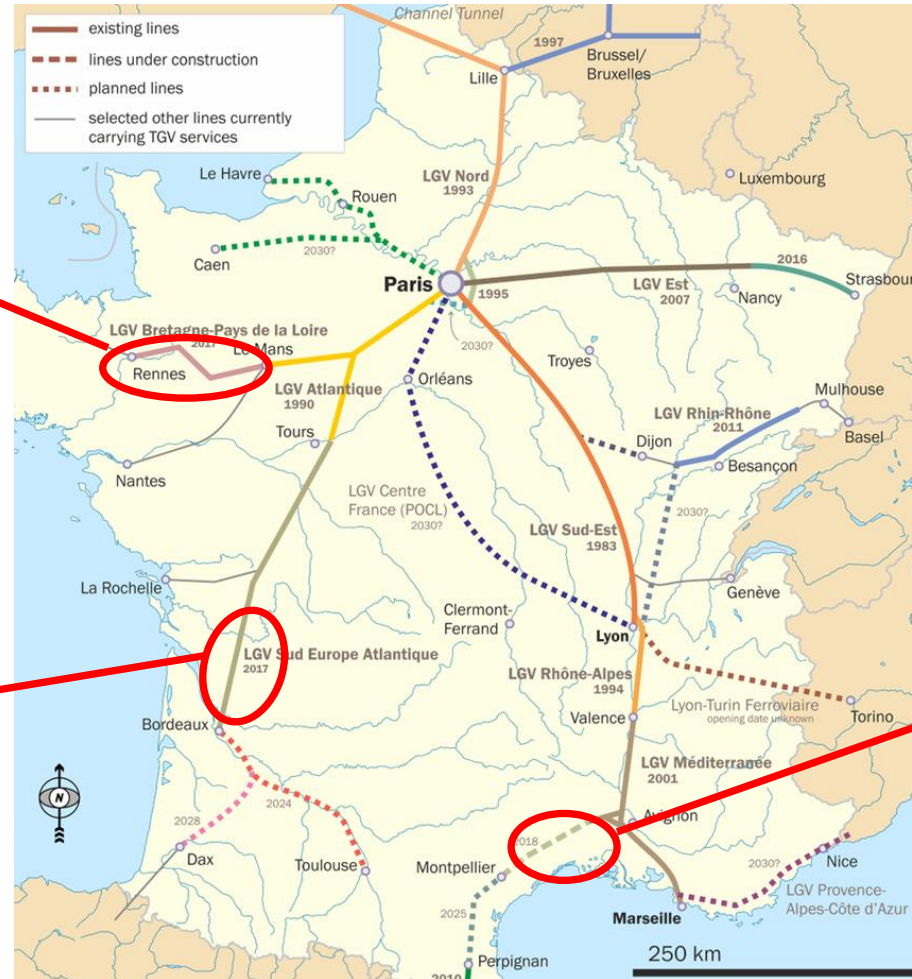
Comparison between
FEM and measurements
(from D Ramirez Cardona Thesis)

Large recent projects in France

BPL
105 km

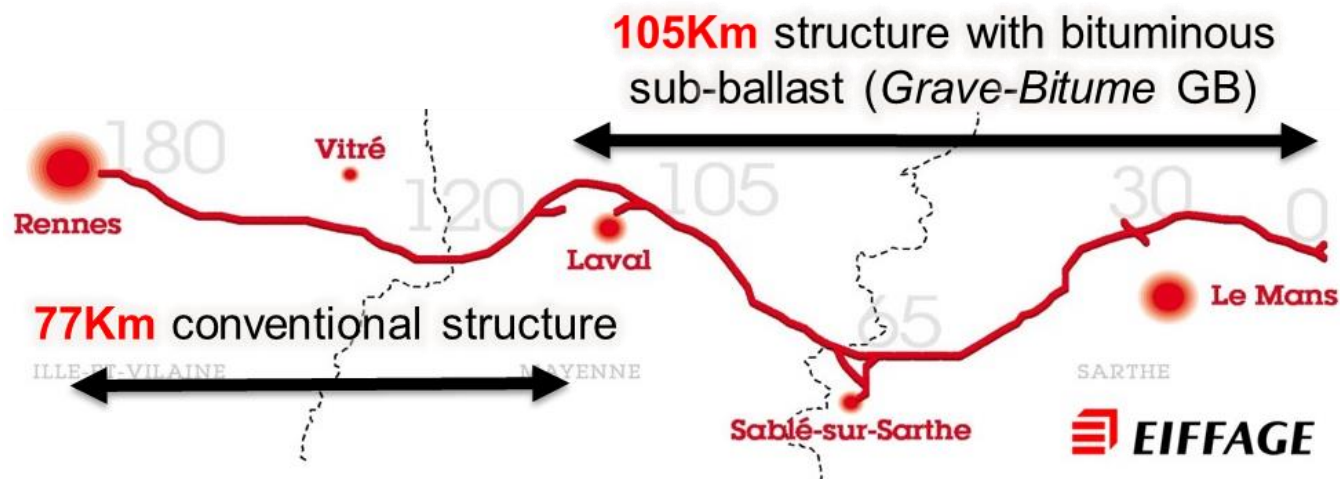


SEA
43km



CNM
80 km





4

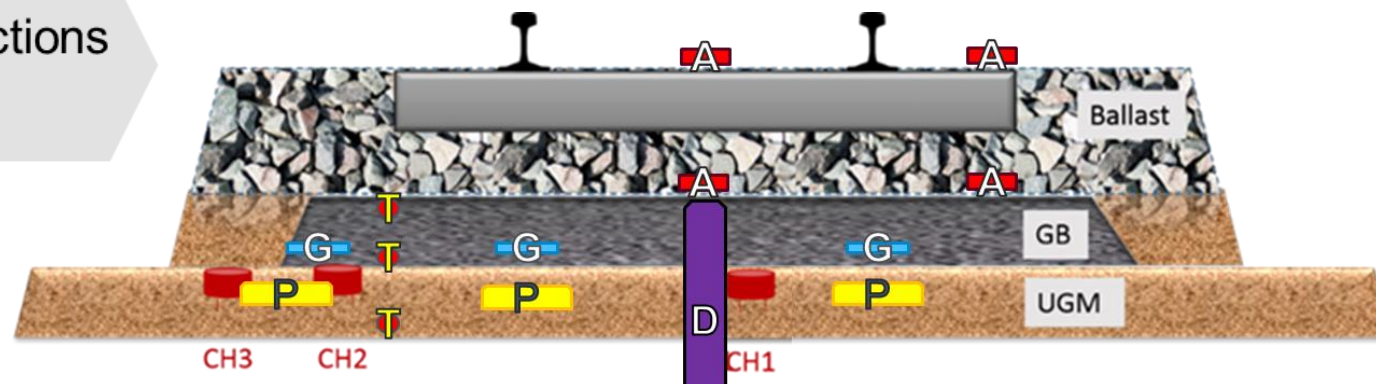
Fully instrumented sections for track monitoring

265k

Tons of GB4


12

Months of work




 Vertical stress

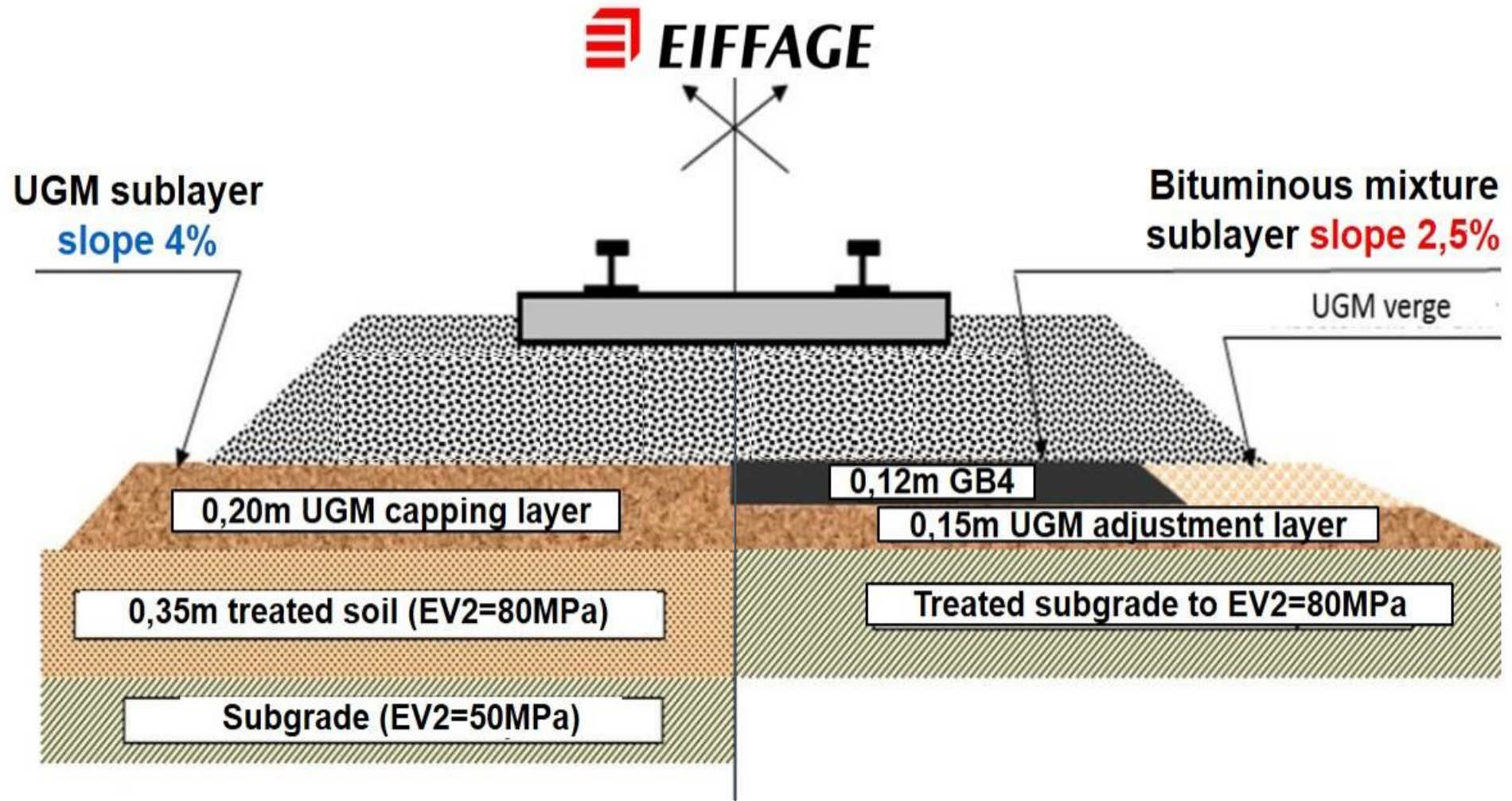
 Temperature

 Transv. & long. strain

 Acceleration

 Water content

 Track deflection

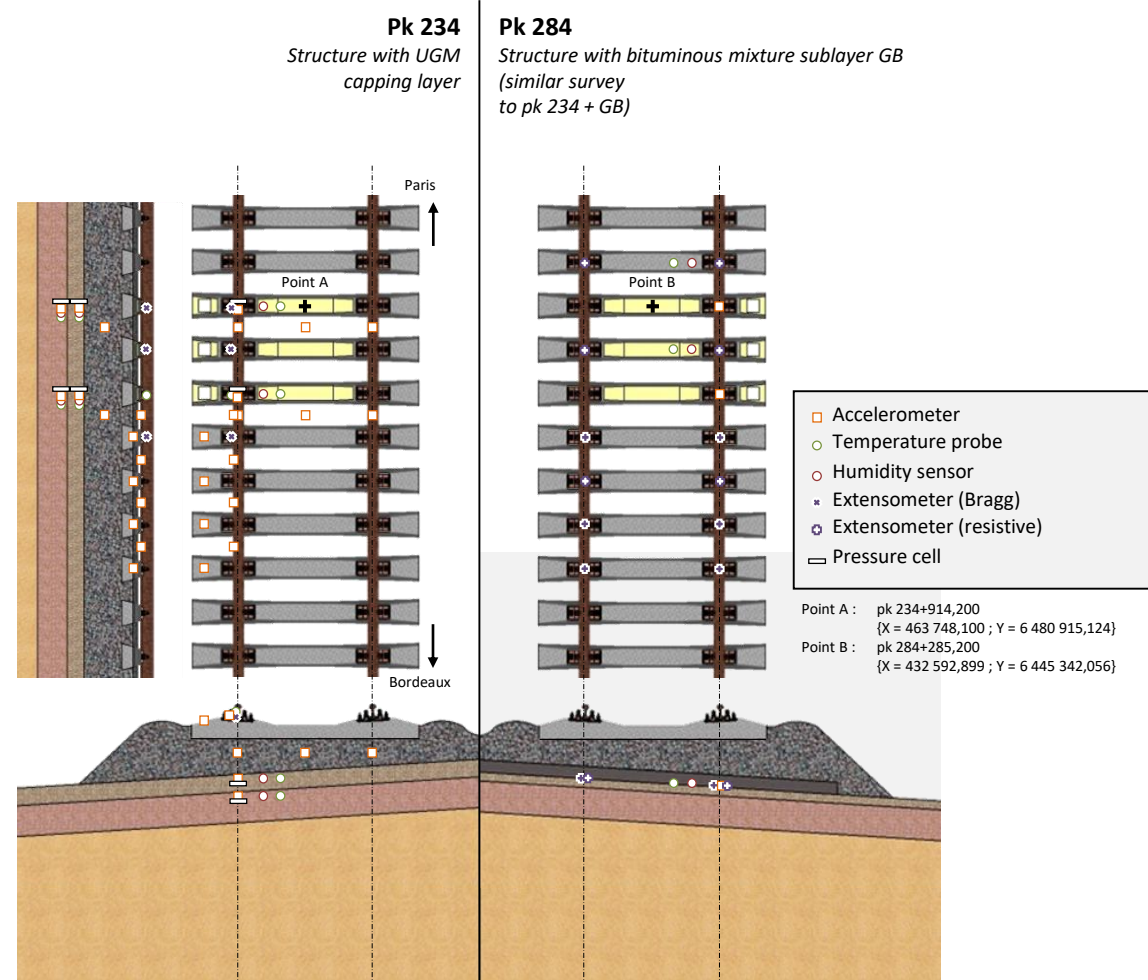
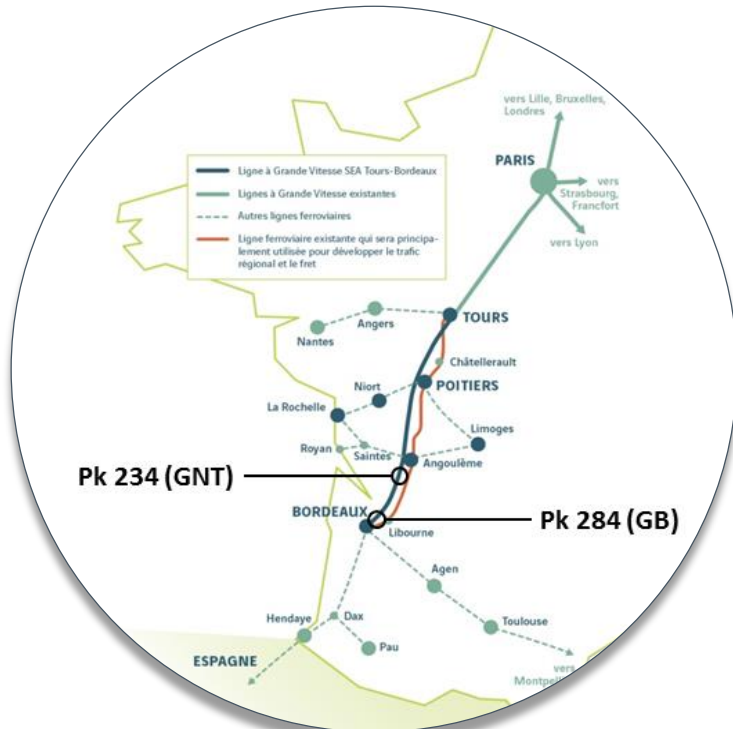


SEA

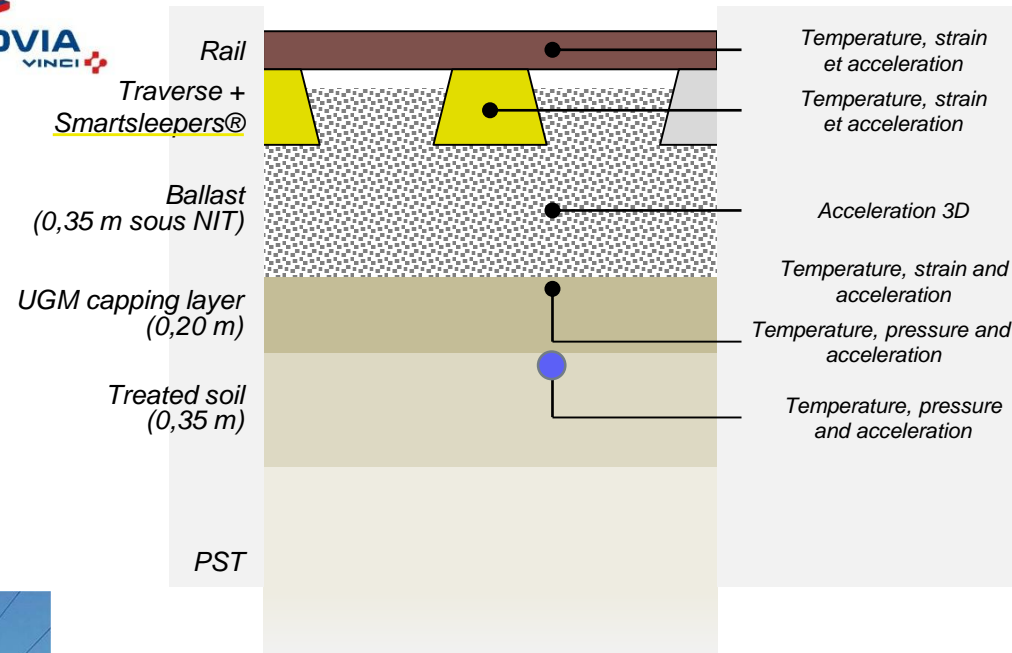
Cosea
CONSTRUCTION SEA TOURS - BORDEAUX



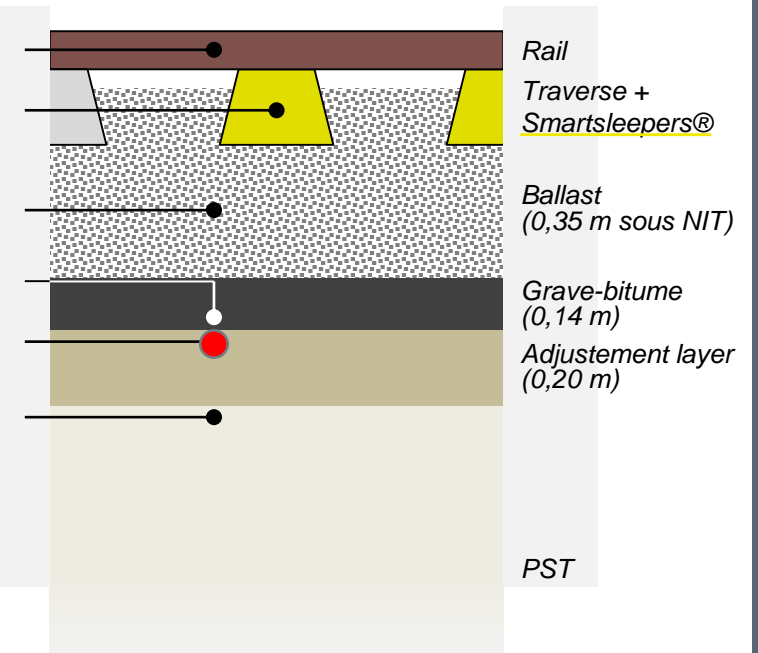
- 50 years concession
- 43 km (over 340 km) of “GB” (south of the line)
- Year of commissioning of the railway line: 2017



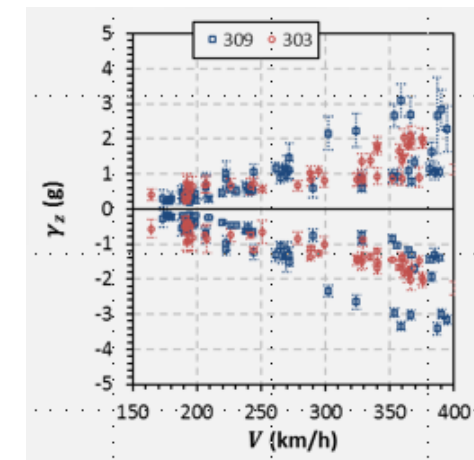
Pk 234 (structure avec GNT)



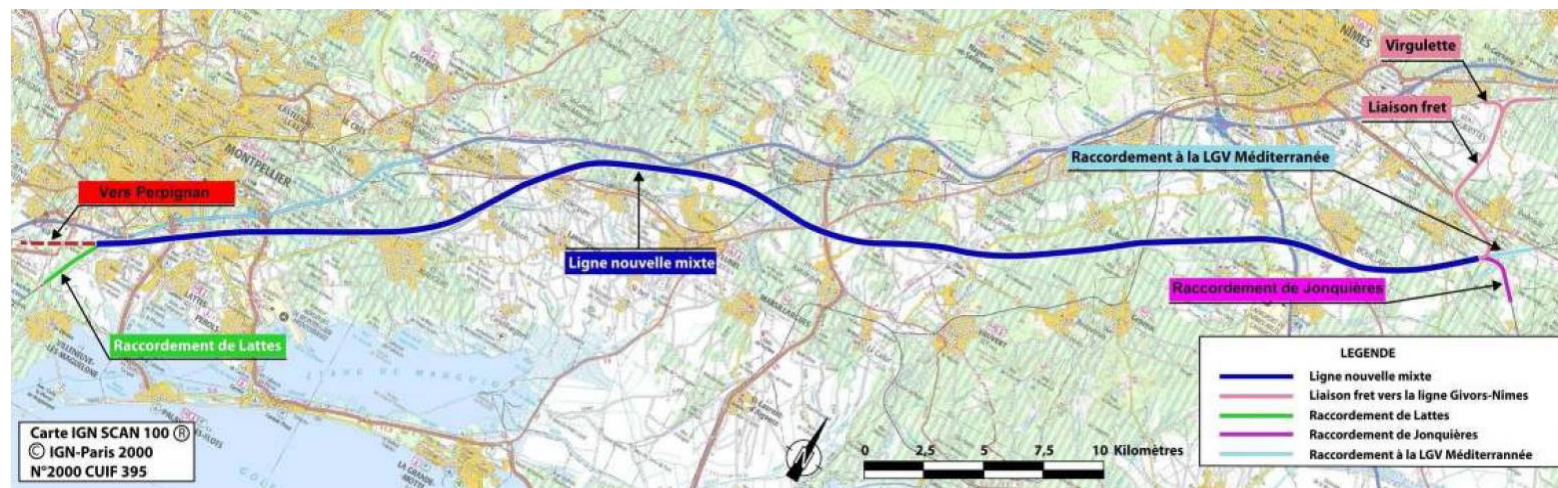
Pk 284 (structure avec GB)



- From a model approach (*Martin et al. 2017*), acceleration γ_z for asphalt structure (15 cm; 15 °C) and $V = 342 \text{ km/h} \sim \gamma_z^{ref}$ is equivalent to and $V = 270 \text{ km/h}$ for a conventionnal structure .
- The trend is confirmed by onsite feedback.



CNM



44 years Public Private Partnership

*High speed track 61 km
+ 21 km connections*






Mixed traffic : HST + Freight

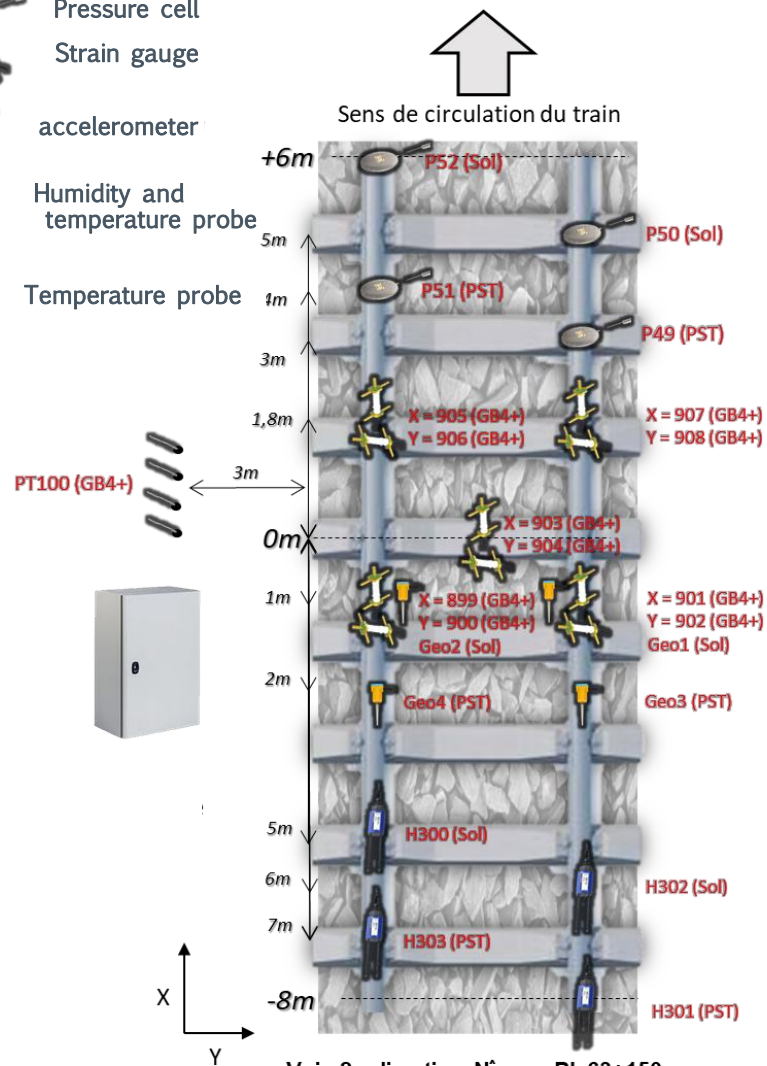
239 000 t Base asphalt GB4+

	Requirements
Binder content	$k > 2,9$
Voids	$\leq 4\%$
Modulus	$\geq 11000 \text{ MPa (3-6\% voids)}$
Fatigue	$\geq 110 \mu\text{defs (3-6\%voids)}$

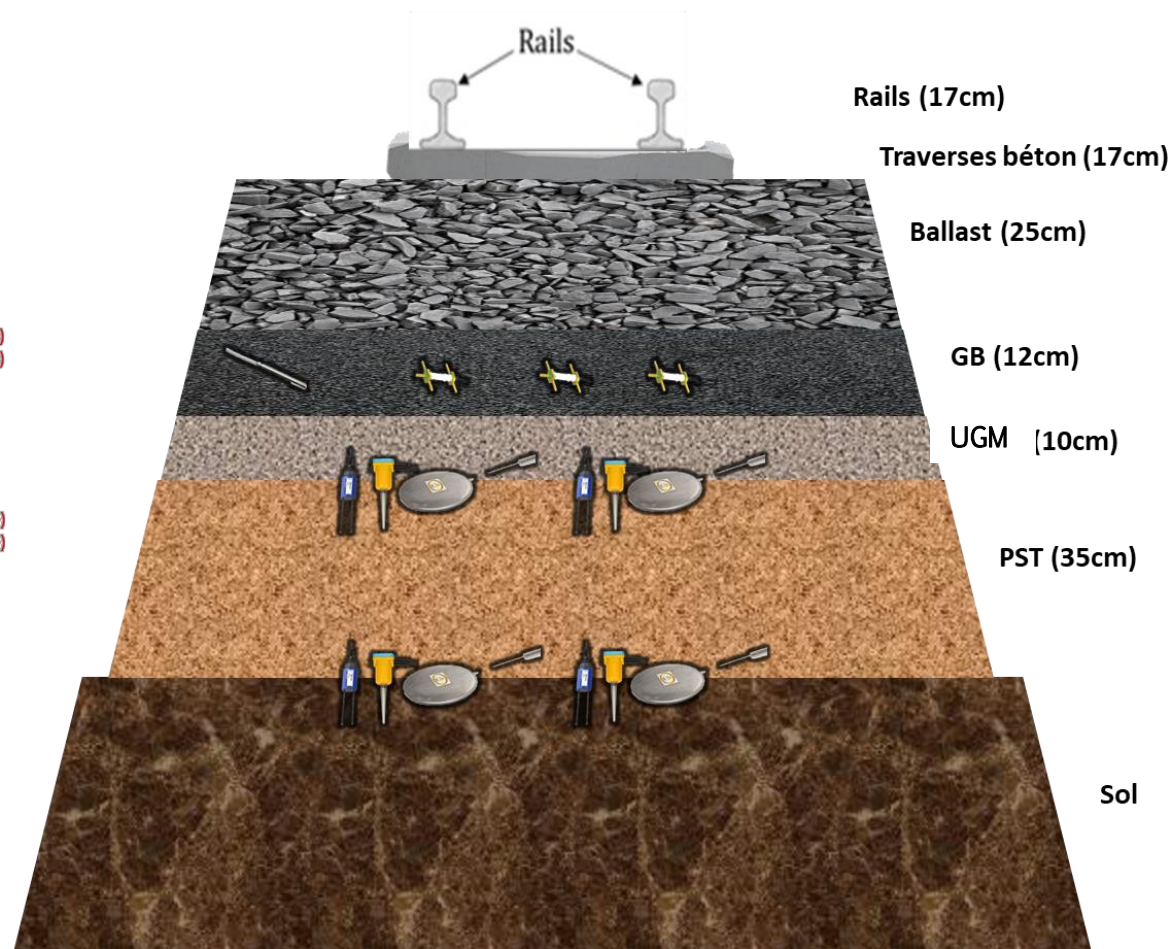
Base course covered by emulsion ECR 65% 800g/m² + gritting 4/6 6l/m²

CNM COLAS

-  Pressure cell
-  Strain gauge
-  accelerometer
-  Humidity and temperature probe
-  Temperature probe

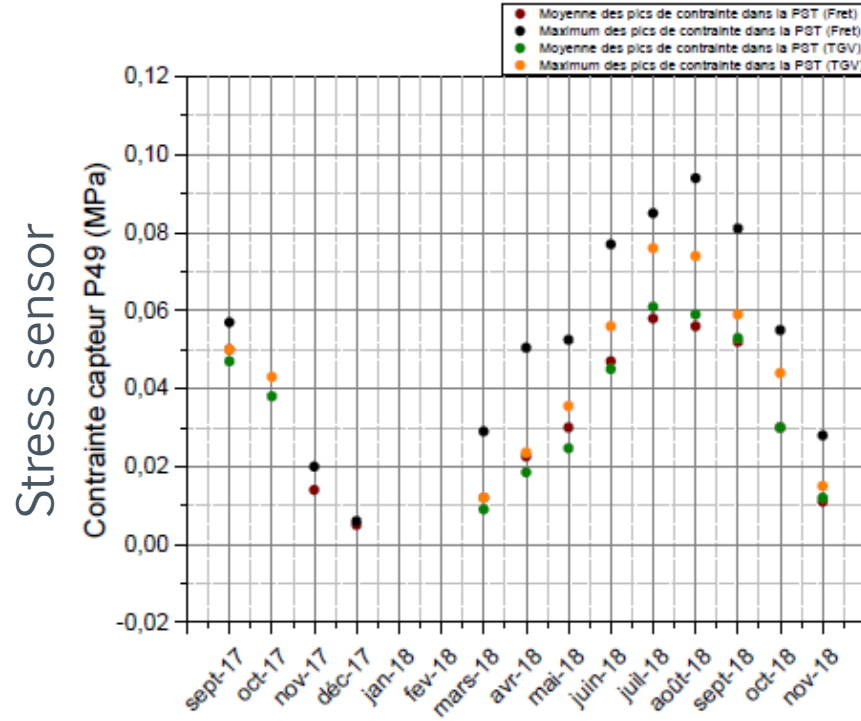


Voie 2 : direction Nîmes, Pk 63+150

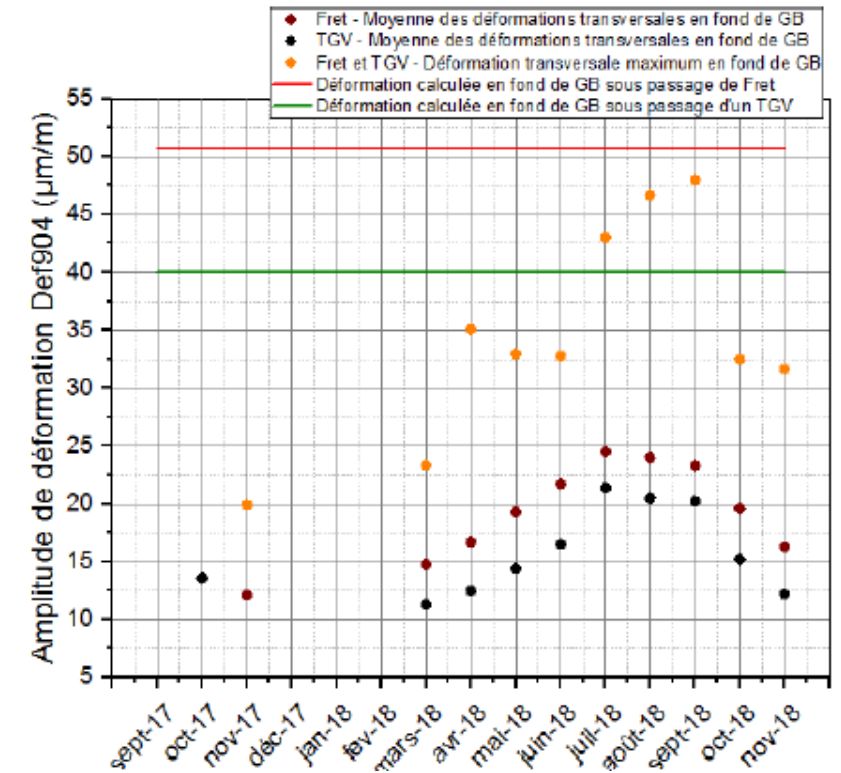


Feedback from Projects & Monitored sections

- › Validation of assumptions used for design
- › Measures lower than maximum values



Strain amplitude (bottom GB)



Ex. measures from CNM HSL

Feedback from Projects & Monitored sections

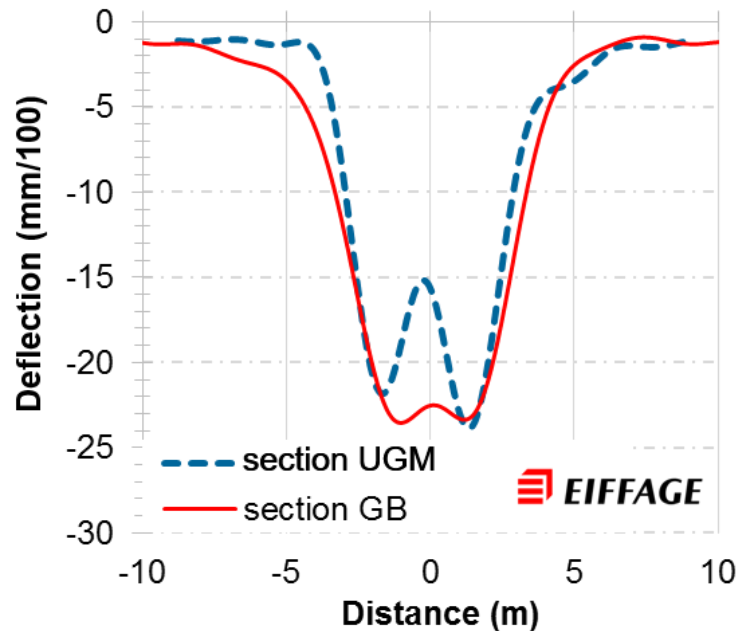
› Positive effect on vertical acceleration



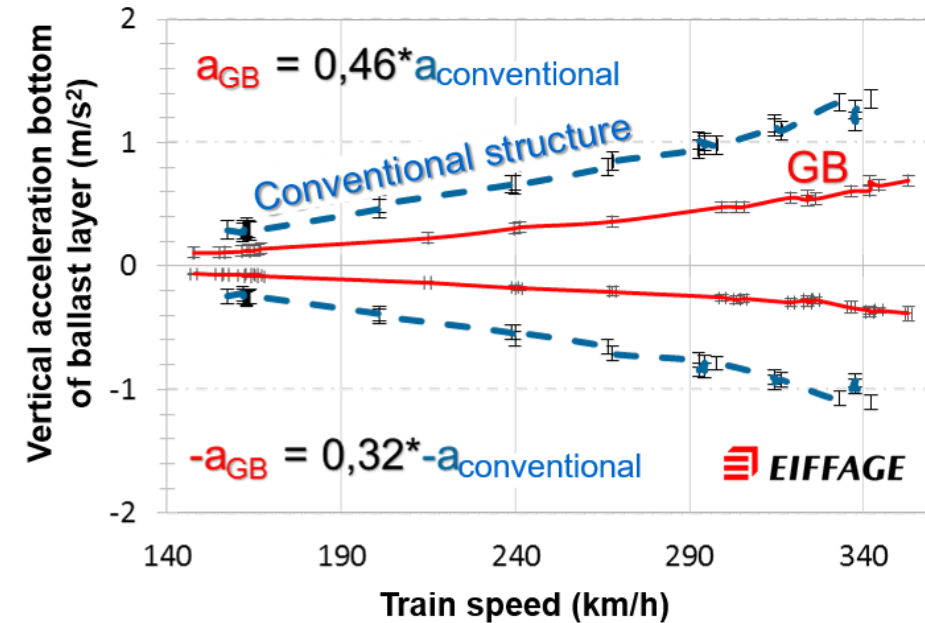
Khairallah et al. 2018

PhD thesis in progress
(due for 10/19)

- Deflexion: Similar magnitude but traction return attenuated



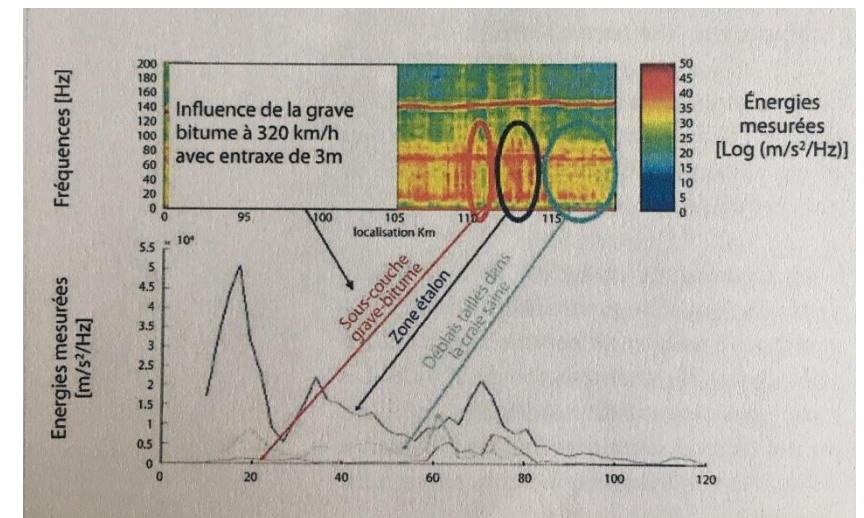
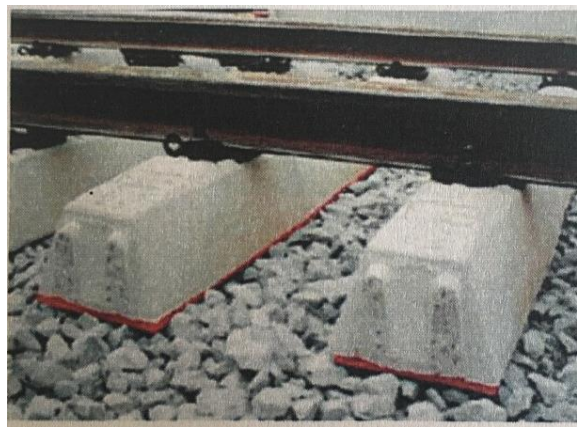
- Vertical acceleration: 40% less at the ballast bottom and 50% less on the sleeper (320km/h train speed)



Ex. measures from BPL HSL

Feedback from Projects & Monitored sections

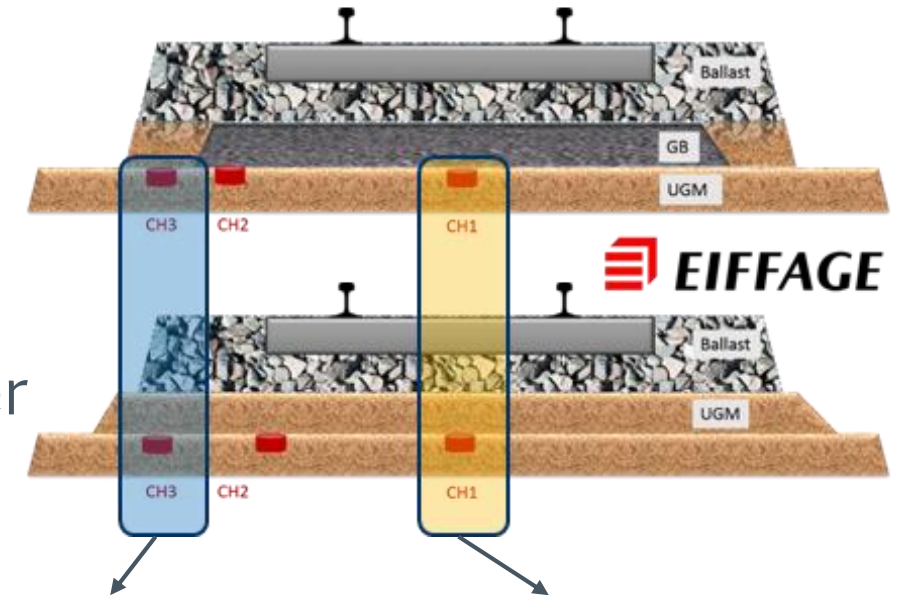
- › Benefit for longitudinal levelling
 - Reduction of maintenance cost due to lesser need for tamping
- › Good solution for a speed of 320km/h
 - Monobloc concrete sleepers + pads to reduce attrition
 - Asphalt sub-ballast trackbed



Feedback from Projects & Monitored sections

- › Platform protection from water infiltration
 - Stable moisture content of the soil layers
 - Increased durability of the platform
 - Enhanced evacuation of run-off water

Moisture probes in the track



Structure type	Rain time	Rainfall	Subgrade water content variation	
			Ballast limit	Track axle
All UGM	12h	23mm	26%	9%
GB4 sub-ballast	14h	23mm	13%	0%

Ex. measures from BPL HSL

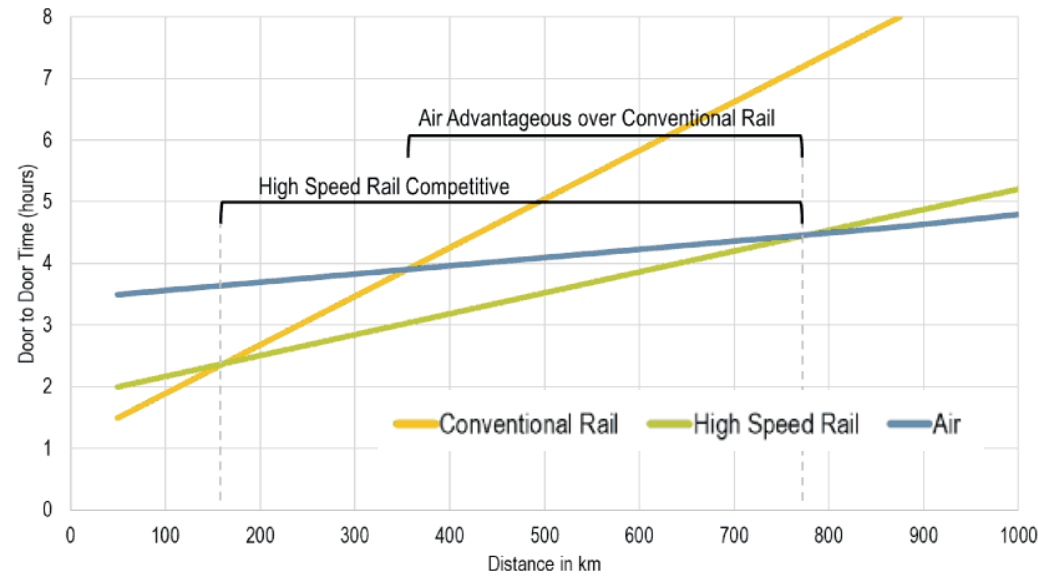
Feedback from Projects & Monitored sections

Summary:

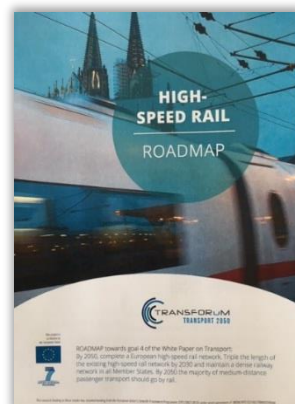
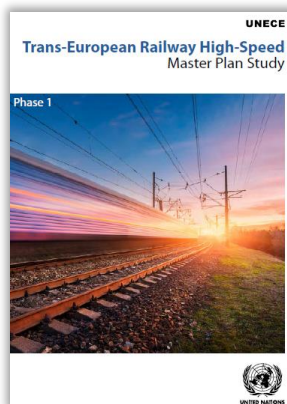
- › Good behaviour of GB
- › Positive effect on deflection and acceleration under Ballast
- › Good solution for High speed (320 km/h)
- › As a result : Savings in maintenance
- › Prevent damage due to water (durability).
- › Care with drainage and design for remediation.

ASPHALT IN RAILWAYS

OPPORTUNITIES FOR THE FUTURE?



Source : Commission for Integrated Transport, London (2004) High-speed rails: international comparisons, Steer Davies Gleave.



1000km radius = ~
5h door-to-door from Paris
HST = ~ Airplane